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Reactions and free-body (or equilibrium) diagrams.



Types of support conditions: idealized models.

Type of connection	Typical symbols	Types of trans- lations and rota- tions the connection allows	Type of forces that can be developed at the connection	Types of forces that can be developed when the support is inclined
Fixed support				
Pinned support				
Roller support				90°
Simple support				90°
Cable support	↓		<u>↓</u>	

Reference: Structures, Daniel Schodek, Martin Bechthold.

Nonrigid and rigid structures.



(a) Rigid structure (e.g., a beam). The structure is stiff and does not undergo appreciable changes in shape with changes in the loading condition.



(b) Nonrigid or flexible structure (e.g., a cable). The shape of the structure changes with changes in the loading condition.

Types of structural elements.



Types of structural elements.



Types of structural elements.



Typical structural units.



(a) Common types of horizontal spanning systems (one-, two-, and three-level systems) used in relation to different types of load-bearing wall and columnar vertical support systems.

Typical structural units.



- (b) Common assembly of elements. The surface-forming decking is supported by a secondary framing system consisting of closely spaced beams, which is in turn supported by a primary system of more widely spaced trusses. A one-on-one fit exists between the trusses and the supporting columns.
- (c) The decking transfers roof loads to the secondary beams. The beams transfer the loads to the trusses, which in turn carry them to the columns. The columns transmit the loads to the foundations. These force transfers occur through the development of reactive forces between members and typically get progressively larger at lower levels.

Dominant stress states: bending, shear, and axial forces and stresses in common structural forms under primary loadings.



Beams and frames: primarily in bending, with shear and axial forces



Trusses: axial tension (T) and compression (C) in members



Cables:

tension (T)

Arches: compression (C) under primary loadings



Plates: primarily biaxial bending, with shear



Space frames: axial tension (T) and compression (C) in members



Folded plates: beam-like action in bending, with shear



Pneumatic structures: in-plane biaxial tension membrane stresses in surface



Membrane structures: in-plane biaxial tension membrane stresses in surface, masts in compression, and tie-backs in tension



in-plane membrane

stresses in surface

at boundaries

Shells:

Free-form rigid shapes: primarily in bending with some (T or C), minor bending in-plane tension or compression

Table 2.1(a)Support conditions for coplanar structures.

Idealized Symbol	Reactions	Number of Unknowns
SMOOTH SVPFACE	P P	One unknown reaction. Reaction is perpendicular to the surface at the point of contact.
SHORET SHORET SHORET LINE	The second secon	One unknown reaction. The reaction force acts in the direction of the cable or link.

Table 2.1(b) Supports and connections for coplanar structures.

Idealized Symbol	Reactions	Number of Unknowns
POLLER POOLER POOLER		One unknown reaction. Reaction is perpendicular to the supporting surface.





Table 2.2 Connection and support examples.









(a) Pictorial diagram.



(b) Free-body diagram of the beam.

Figure 2.53 Simple beam with two concentrated loads.

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(a) Pictorial diagram.

(b) Free-body diagram.

Figure 2.54 Cantilever beam with a concentrated and uniform load.



(a) Pictorial diagram.

(b) Free-body diagram.

Figure 2.55 Wind load on a pitched roof. Wind loads on pitched roofs are generally applied perpendicular to the windward surface. Another analysis would examine the uplift forces on the leeward slope. Purlins that run perpendicular to the plane of the truss are generally located at the truss joints to minimize bending in the top chord member.





2.28 Draw an FBD of member *ABD*. Solve for support reactions at *A* and the tension in cable *BC*.

Solution:

The directions for A_x , A_y , and *BC* are all assumed. Verification will come through the equilibrium equations.

$$[\Sigma M_A = 0] + 0.707BC(2.5 \text{ m}) - 2.4 \text{ kN}(5 \text{ m}) = 0 \quad (1)$$

$$BC = \frac{2.4 \text{ kN}(5 \text{ m})}{0.707(2.5 \text{ m})} = 6.79 \text{ kN}$$

$$\therefore BC_x = 0.707(6.79 \text{ kN}) = 4.8 \text{ kN}$$

$$\therefore BC_y = 0.707(6.79 \text{ kN}) = 4.8 \text{ kN}$$

$$[\Sigma F_x = 0] + A_x - 4.8 \,\mathrm{kN} = 0 \tag{2}$$

$$A_x = +4.8 \text{ kN}$$
$$[\Sigma F_y = 0] - A_y + 4.8 \text{ kN} - 2.4 \text{ kN} = 0$$
(3)
$$A_x = +2.4 \text{ kN}$$

Reactions for a simple beam (basit kiriş) with vertical loads.



Reactions for a cantilevering (overhang) beam / çıkmalı kiriş loaded with a point load.



Reactions for a cantilevering (overhang) beam loaded with two point loads.



Reference: Structures, Daniel Schodek, Martin Bechthold.

Reactions for an L-shaped beam with horizontal and vertical point loads.



Beam with roller resting on a sloped support.



Simply supported beam (basit kiriş)



Reaction for a truss-like structure.





Moment about *B* produced by external forces

Resisting moment that must be developed by the reaction

Moment about *A* produced by external forces

Resisting moment that must be developed by the reaction The directions of the reactions can be determined by inspecting how the external forces rotate the body.

Fixed-end moments.



Determination of the loading model for a cable-supported structure. The analysis is simplified and based on several assumptions.



Load modeling is discussed extensively in Chapter 3.

Cable-supported structure.





Figure 2.56(a) Truss with hinge and roller support.



Figure 2.56(b) FBD—Determinate and constrained.



Figure 2.57(a) supports.





Figure 2.57(b) FBD—Statically indeterminate externally.



(a) Pictorial diagram.



(b) Free-body diagram.

Figure 2.58 Two rollers—partially constrained/unstable.



(a) Pictorial diagram.



(b) Free-body diagram. Figure 2.59 Two rollers—partially constrained/unstable.



(a) Pictorial diagram.



Figure 2.60 Three supports—stable and determinate.

Three Equations of Equilibrium and Three Unknown Support Reactions

Classification of Structures Based on Constraints			
Structure	Number of Unknowns	Number of Equations	Statical Condition
1. Fi F ²	3	3	Statically determinate (called a <i>simple beam</i>)
2.	5	3	Statically indeterminate to the second degree (called a <i>continuous beam</i>)
3.	2	3	Unstable
4. Fi	3	3	Statically determinate (called a <i>cantilever</i>)
5. F. F.	3	3	Statically determinate (called an <i>overhang beam</i>)

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Three Equations of Equilibrium and Three Unknown Support Reactions

	Structure	Number of Unknowns	Number of Equations	Statical Condition
6.	Fi Manthank	4	3	Statically indeterminate to the first degree (called a <i>propped beam</i>)
7.	F F2	6	3	Statically indeterminate to the third degree (called a <i>fixed-ended beam</i>)
8.	Fi TH-USS	3	3	Statically determinate (see Example Problem 3.8)
9.	TR-VSS	3	3	Unstable (improperly constrained)
10.	P-IG-ID FRAME	6	3	Statically indeterminate to the third degree

LOAD SYSTEMS

Distributed loads, as the term implies, act on a relatively large area—too large to be considered as a point load without introducing an appreciable error. Examples of distributed loads include:

- Furniture, appliances, people, etc.
- Wind pressure on a building.
- Fluid pressure on a retaining wall.
- Snow load on a roof.

Uniform distributed load (düzgün yayılı yük)





Figure 3.22 Uniformly distributed load with an FBD of the steel beam.

Point or *concentrated loads* have a specific point of application, whereas *distributed loads* are scattered over large surfaces. Most common load conditions on building structures begin as distributed loads. (See Figures 3.20 to 3.24.)



Figure 3.20 Single concentrated load, with an FBD of the steel beam.

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Point or *concentrated loads* have a specific point of application, whereas *distributed loads* are scattered over large surfaces. Most common load conditions on building structures begin as distributed loads. (See Figures 3.20 to 3.24.)



Figure 3.21 Multiple concentrated loads, with an FBD of the wood girder.





Figure 3.23 Uniformly distributed load, with a concentrated load, with an FBD of the steel beam.



Figure 3.25 Equivalent load systems.

The location of the equivalent concentrated load is based on the centroid of the load area. By geometric construction, the centroids of two primary shapes are shown in Figures 3.26 and 3.27.

Resultant Force (Bileşke kuvvet)



Figure 3.26 Centroid of a rectangle (Area = $b \times h$).

Resultant Force (Bileşke kuvvet)



Figure 3.27 Centroid of a triangle (Area = $1/2 \times b \times h$).