

Summary of Structure Equations

n : total number of DOF in the structure

m : unconstrained DOF

k : constrained DOF

$$n = m + k$$

\mathbf{F} is the $n \times 1$ force vector

\mathbf{D} is the $n \times 1$ displacement vector

\mathbf{K} is the $n \times n$ stiffness matrix

\mathbf{P} is the $n \times 1$ fixed end force vector

$$\mathbf{F} = \begin{bmatrix} F_1 \\ F_2 \\ \dots \\ F_n \end{bmatrix}, \quad \mathbf{D} = \begin{bmatrix} D_1 \\ D_2 \\ \dots \\ D_n \end{bmatrix}, \quad \mathbf{P} = \begin{bmatrix} P_1 \\ P_2 \\ \dots \\ P_n \end{bmatrix}, \quad \mathbf{K} = \begin{bmatrix} K_{11} & K_{12} & \dots & K_{1n} \\ K_{21} & K_{22} & & \\ \dots & & \dots & \\ K_{n1} & & & K_{nn} \end{bmatrix}$$

$$\mathbf{F} = \mathbf{K}\mathbf{D} + \mathbf{P}$$

$$\begin{bmatrix} F_1 \\ F_2 \\ \dots \\ F_n \end{bmatrix} = \begin{bmatrix} K_{11} & K_{12} & \dots & K_{1n} \\ K_{21} & K_{22} & & \\ \dots & & \dots & \\ K_{n1} & & & K_{nn} \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ \dots \\ D_n \end{bmatrix} + \begin{bmatrix} P_1 \\ P_2 \\ \dots \\ P_n \end{bmatrix}$$

$$F_1 = K_{11}D_1 + K_{12}D_2 + \dots + K_{1n}D_n + P_1$$

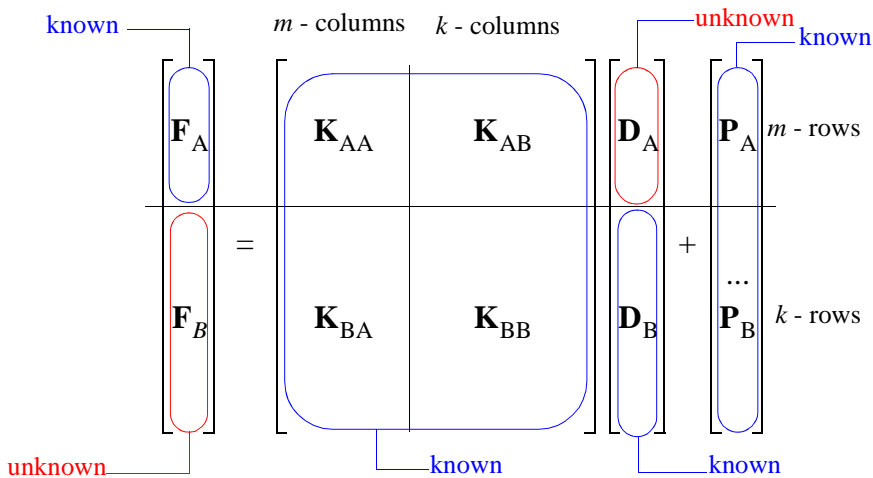
$$F_2 = K_{21}D_1 + K_{22}D_2 + \dots + K_{2n}D_n + P_2$$

...

$$F_n = K_{n1}D_1 + K_{n2}D_2 + \dots + K_{nn}D_n + P_n$$

Partitioning

$$\begin{array}{c}
 \begin{matrix} m - \text{columns} & k - \text{columns} \\
 \begin{bmatrix} F_1 \\ F_2 \\ \vdots \\ F_n \end{bmatrix} = \begin{bmatrix} K_{11} & K_{12} & \dots & K_{1n} \\ K_{21} & K_{22} & & \\ \vdots & \vdots & \ddots & \vdots \\ K_{n1} & & & K_{nn} \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ \vdots \\ D_n \end{bmatrix} + \begin{bmatrix} P_1 \\ P_2 \\ \vdots \\ P_n \end{bmatrix} \\
 \begin{matrix} m - \text{rows} \\ k - \text{rows} \end{matrix}
 \end{matrix}
 \end{array}$$



$$\begin{bmatrix} \mathbf{F}_A \\ \mathbf{F}_B \end{bmatrix} = \begin{bmatrix} \mathbf{K}_{AA} & \mathbf{K}_{AB} \\ \mathbf{K}_{BA} & \mathbf{K}_{BB} \end{bmatrix} \begin{bmatrix} \mathbf{D}_A \\ \mathbf{D}_B \end{bmatrix} + \begin{bmatrix} \mathbf{P}_A \\ \mathbf{P}_B \end{bmatrix}$$

$$\mathbf{F}_A = \mathbf{K}_{AA} \mathbf{D}_A + \mathbf{K}_{AB} \mathbf{D}_B + \mathbf{P}_A \quad (1)$$

$$\mathbf{F}_B = \mathbf{K}_{BA} \mathbf{D}_A + \mathbf{K}_{BB} \mathbf{D}_B + \mathbf{P}_B \quad (2)$$

Note that when there is no support settlement and support rotation, $\mathbf{D}_B = \mathbf{0}$ (displacement and rotations at unconstrained DOF).

Solution of the Equations

(1) gives $\mathbf{D}_A = \mathbf{K}_{AA}^{-1}(\mathbf{F}_A - \mathbf{K}_{AB}\mathbf{D}_B - \mathbf{P}_A)$ (displacements and rotations at unconstrained DOF)

(2) gives $\mathbf{F}_B = \mathbf{K}_{BA}\mathbf{D}_A + \mathbf{K}_{BB}\mathbf{D}_B + \mathbf{P}_B$ (support reactions - reactions at constrained DOF)

Member end displacements in global coordinates, \mathbf{d} , can be found from \mathbf{D} .

Member end displacements in local coordinates and member end forces in local and global coordinates can be found using \mathbf{d} .